STRENGTHEN ALABAMA HOMES CONTRACTOR BID SHEET

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Date

Owner Name

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NEEDS ETROFIT	BID ITEM	WORK DESCRIPTION	BID PRICE
YES NO	Gable Bracing Retrofit	Retrofit Requirements Prescriptive methods for retrofitting gables 4 feet tall and taller are detailed in Appendix A. These methods are intended for applications where the gable end wall framing is provided by a wood gable end truss or a conventionally framed rafter system. These prescriptive methods of retrofitting are intended to increase the resistance of existing gable end construction to out of plane wind loads.	Total Cost
		Four issues are addressed:	
		1. Strengthening the vertical framing members of the gable end with the use of retrofit studs;	
		2. Bracing the top and bottom of the gable end so the lateral loads are transmitted into the roof and ceiling diaphragms through horizontal braces;	
		3. Making connections between horizontal braces and retrofit studs using metal straps and fasteners; and,	
		4. Connecting the bottom of the gable end to the wall below using metal bracket connectors.	
		Note: Performing the retrofits required for strengthening gable ends may be easier and provide easier access to the gable end, if combined with the retrofits for strengthening outlooker connections to the gable end or adding gable wall sheathing, when required for the Hurricane Resistance Bronze Designation.	
		Minimum requirements for use of prescriptive methods detailed in Appendix A:	
		1. Minimum ceiling diaphragm must be ½-inch drywall, 3/8- inch thick plywood, or plaster installed over wood lath.	
		2. Minimum roof diaphragm must be 7/16-inch plywood or OSB.	
		3. Gable ends must have structural wall sheathing (minimum of 7/16-inch plywood or OSB or equivalent).	
		Cases that are not covered in this retrofit guidance require that a licensed professional engineer design a gable end bracing system that will meet wind forces appropriate for the location.	
		Note: Gable ends that are not covered in this retrofit guidance include:	
		1. Gable end walls on rooms with vaulted or cathedral ceilings	
		2. Gable ends taller than 16 feet and/or have irregular shape.	
		*See Pages 31 nad 32 and Appendix "A" of the FFH Engineering Guide	
YES	OPENING PROTECTION	All Openings must be protected with products having a Florida Building Code or Miami-Dade Product	
NO		Approval for Cyclic and Large Missile Impact (9-lb) Products as indicated below, Products used shall bear a lable indicating the manufacturer, testing complinace method, and Product Approval Number.	
		All Glazed Windows/Entry Doors/Garage Doors and All Non-Glazed Entry Doors are reqquired to be impact reistant or protected with an impact protective covering. All Non Glazed garage doors are required to meet site specific design pressures for wind load resitance.	#/\$ Windows/Glaze Doors
		Approved Testing Standards are:	#/\$ NonGlazed Entry
		FBC or MDC TAS 201, 202, and 203 ASTM D 1886 AND ASTM D 1996 Missile D (for Windzone 3 or 4)	#/\$ Glazed Garage
		SBCCI SSTD-12 (9lb ofr Wind Zone 3) For Non-Glazed garage Doors ANSI/DASMA 108 or AST M E 330, OR TAS 202 For Glazed garge doors only ANSI/DASMA 115 For Skylinghts only. ASTM F 1886 and ASTM E 1996 Missile C. (for Windtone 3 or 4)	#/\$ Non-Glazed Ga Doors
		For Skylights only- ASTM E 1886 and ASTM E 1996 Missile C (for Windzone 3 or 4)	

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NEEDS		BID ITEM	WORK DESCRIPTION	BID PRICE	
		lumn Load Path Retrofit	These covered, attached structures are usually supported by horizontal beam members sitting on	Total Cost	
NO			vertical columns, which are then connected to foundation systems. Improving the anchorage of these structures requires three steps:		
			1. Provide metal connectors between the supporting roof members and the horizontal beams. It may be necessary to remove soffit/ceiling material in order to reinforce the connection. The uplift load on this connection can be determined by completing the Uplift Worksheet.		
			a. Wood to Wood connections: The saddle-type hurricane clip (e.g. H10 or HS10 type clips) may be installed on either side of the beam when the determined uplift force is less than 800 lbs and must be installed on both sides of the beam when the determined uplift is greater than 800 lbs.		
			2. Provide a metal connector at the top of each beam to column connection. The uplift load required for this connection can be determined by completing the Uplift Worksheet. Select one of the connections shown in Figure VI-2. The determined uplift force must be smaller than the stated allowable uplift capacity corresponding to the selected connection.		
			a. The metal connector must be rated for exterior weather exposure and the installation must be in accordance with the manufacturer's recommendations.		
			3. Provide a metal connector at each column to foundation connection. The uplift load required for this connection can be determined by completing Uplift Worksheet. Select one of the connections shown in Figure VI-3 so the determined uplift force is less than the corresponding allowable uplift capacity.		
			a. The metal connector must be rated for exterior weather exposure and the installation must be in accordance with the manufacturer's recommendations. Provide a moisture barrier between the bottom of metal connector and the concrete.		
			*See Pages 33 thru 37 of the FFH Engineering Guide		
PORCH/CA	ARPORT	Use the following guideling	nes to determine how much uplift resistance is required to retrofit the existing carport/porch co	olumn connection at	
COLUMN	UPLIFT		A continuous load path must be achieved from the roof framing members to the supporting b	eam, from the beam	
WORKSHE	to the column, then from		n the column to the foundation.		
		I. Measure how far the porch roof sticks out from the wall, D =ft.			
		II. Measure the width of the porch parallel to the house wall, W = ft.			
		III. Measure the roof member spacing, S = ft.			
		IV. Measure the roof ove	rhang distance, OH = ft.		
		V. Count the number of columns supporting the roof (whole number = N) (Count each end wall as a post that supp instead of a post, maximum 2.)		ports the roof	
		VI. Column support area can be calculated as following:			
		Inside Column Area A= × Corner Column Area A= ×			
		VII. Select the appropriate net uplift pressure (wind pressure minus weight) for the design wind speed at your house from the Uplift Pressure Table below, P = psf.			
	VIII. The roof member uplift		lift force can be calculated as follows:		
	Pup = P * (D/2 + OH) * S =Ibs.				
	typical area, P*A = This is the uplift on each o		e beam to column and column to foundation can be calculated by Multiplying the net uplift prelbs.	essure times the	
			column, on the connection at the top of the column, and also on the connection at the bottom crete or masonry) then you can reduce the force on the connection at the bottom of the colum. mn.		
		*See Pages 36 and 37 of the	e FFH Engineering Guide		